Exhibit H

Florence, OR Dollar General

US Highway 101 Florence, OR 97439

PREPARED FOR



361 SUMMIT BLVD, SUITE 110 BIRMINGHAM, AL 35243

PREPARED BY JSA CIVIL

Engineering | Planning | Management

111 TUMWATER BLVD SE, SUITE C210 TUMWATER, WA 98501 CONTACT: CHARLIE SEVERS, PE PHONE: 360|515|9600

DESIGNER'S CERTIFICATION AND STATEMENT:

I hereby certify that this Stormwater Management Report for Florence Oregon, Dollar General has been prepared by me or under my supervision and meets minimum standards of the City of Florence and normal standards of engineering practice. I hereby acknowledge and agree that the jurisdiction does not and will not assume liability for the sufficiency, suitability, or performance of drainage facilities designed by me.

Charlie Severs, PE Principal Date



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1. PROJECT OVERVIEW AND DESCRIPTION

This Stormwater Management Report was prepared for the proposed commercial development project that will be located at NE INT Oregon Coast Highway & 35th Street Florence, OR. The Stormwater Management Report was prepared to comply with the minimum technical standards and requirements that are set forth in the 2010 City of Florence Stormwater Design Manual. The proposed commercial developed will be constructed on Lane County Tax Parcel No. 1812232206800. Specifically, the proposed site improvements / construction activities include the following:

- Site Preparation, grading, and erosion control activities
- Construction of a 10,460 ft² commercial development retail building
- Construction of an impervious parking lot
- Construction of on-site stormwater facilities
- Extension of utilities (water, power, sewer, etc.)

This existing site is approximately 0.99 acres in size and currently undeveloped in a Highway Commercial H-C zone. The proposed site is bounded by a Burger King restaurant on a parcel directly south, US Highway 101 to the west, a restaurant (Chens Family Dish) to the north and residential homes to the east. The proposed site improvements will disturb the entire site. The project area generally slopes in the north to south direction. According to the National Wetland Inventory mapping center, there are no on-site wetlands. Q

Permits required for this project include City of Florence Type III Planning Commission Review, Oregon Department of Transportation Access Permit, and final building permit.

See Appendix A for Vicinity MAP

2. METHODOLOGY

The existing drainage on site sheet flows from the north side to the south side of the site where it eventually is collected in catch basins and directed to the existing storm water system. Roof runoff and a portion of landscaping runoff north of the proposed building will be routed to a below-grade soakage trench. Parking lot and the remaining landscaping runoff will be conveyed to an at-grade infiltration rain garden.

The geotechnical report concludes that groundwater, generally, is approximately 8' below existing grades. 5' of separation between the bottom of the storm facilities and the groundwater elevation is feasible.

3. ANALYSIS

Using Autodesk's Storm and Sanitary Analysis (SSA). This computer program was used to model, analyze, and design the drainage basin using Santa Barbra UH Hydrology Method and SCS TR-55 TOC Method. The report can be seen in the appendix.

DESIGN ASSUMPTIONS

According to the City of Florence Stormwater manual the presumptive approach will be used for this site. The presumptive approach consists of designing to the 25-year, 24-hour storm stored and infiltrated. After revieing the geotechnical report, the soil infiltration rate that will be used is 4 inches/hour. The curve numbers used are 98 for impervious area and 61 for pervious areas.

CALCULATIONS

Flow Control:

Using the presumptive method Flow Control Storm event is a 25-year, 24-hour event.

The following table identifies the different land-type designation and theory respective areas for the onsite threshold discharge area.

Areas	Curve #	Acres (SF)	Percentage		
Impervious Area	98	0.73 (31,804)	75%		
Pervious Area	61	0.24 (10,370)	25%		
Total	N/A	0.97 (42,174)	100%		

Water Quality:

An infiltration rain garden will be constructed on the east side of the project area for treatment and infiltration of parking lot runoff.

4. ENGINEERING CONCLUSIONS

A SSA model was-up for the threshold discharge area and was completed in accordance with the city's presumptive approach requirements. To retain the 25-year, 24-hour storm event, approximately 3,500 ft³ of storage is required. 4,100 ft³ of storage will be provided. An infiltration rain garden with an overflow to below-grade storage (one connected system) is proposed. Stormwater overflow beyond the 25-year, 24-hour storm event will be collected in an overflow structure and connected to the city's stormwater conveyance system in 35th Street.

5. STORMWATER FACILITY DETAILS/EXHIBITS

See Appendix B for stormwater facility BMP Details

6. OPERATIONS AND MAINTENANCE AGREEMENT

See Appendix A.4 for Operations and Maintenance Plan and O&M Form. The property owner will be responsible for all long-term Operations and Maintenance costs.

Note: O&M manual will be provided with the final report.

7. ADDITIONAL FORMS

Appendix A.3 and A.4 from the Florence Stormwater Design Manual.

APPENDIX A VICINITY MAP







JSACIVIL

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111 Tumwater Blvd SE | Tumwater, WA | 98501

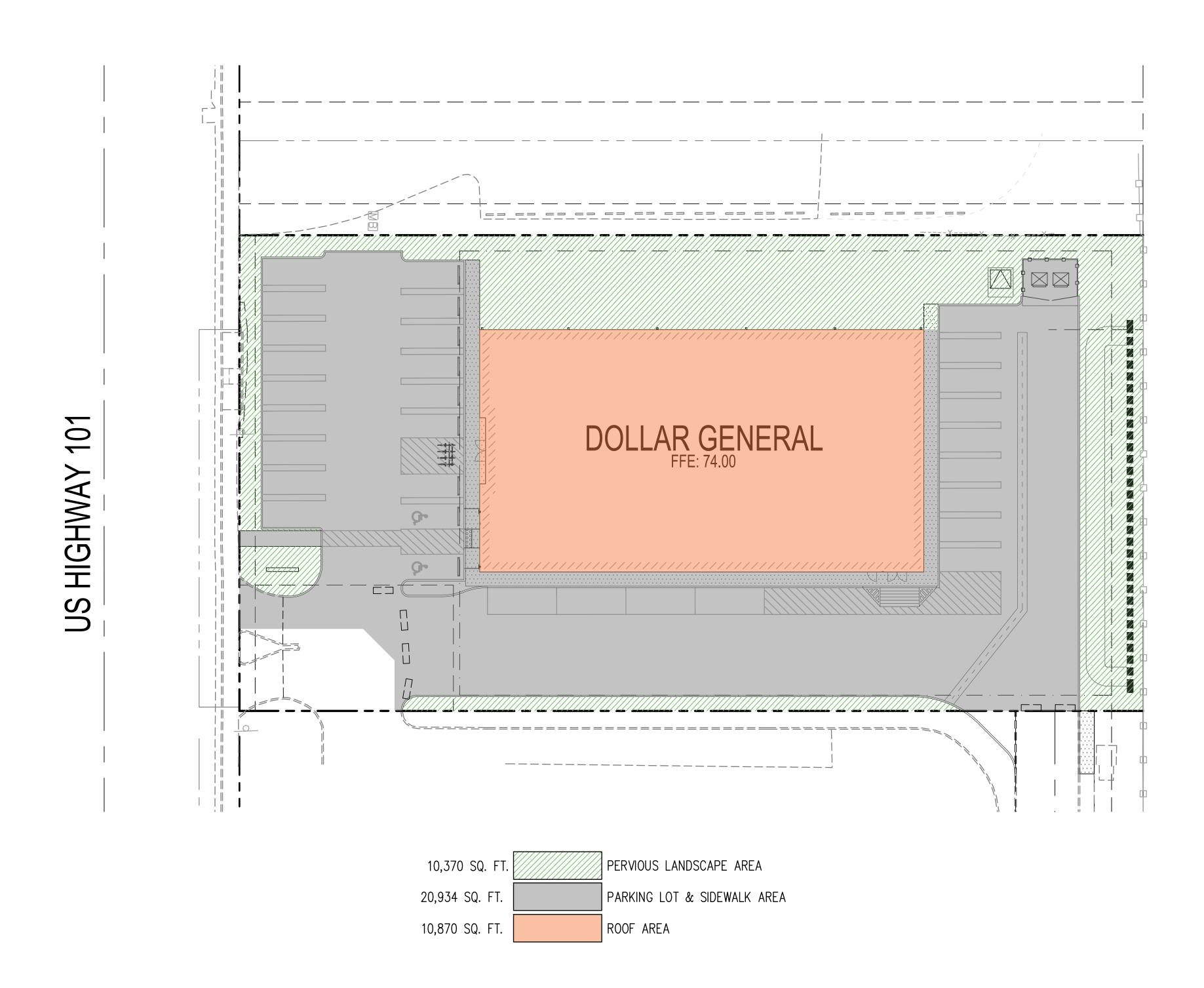
FLORENCE, OR DOLLAR GENERAL

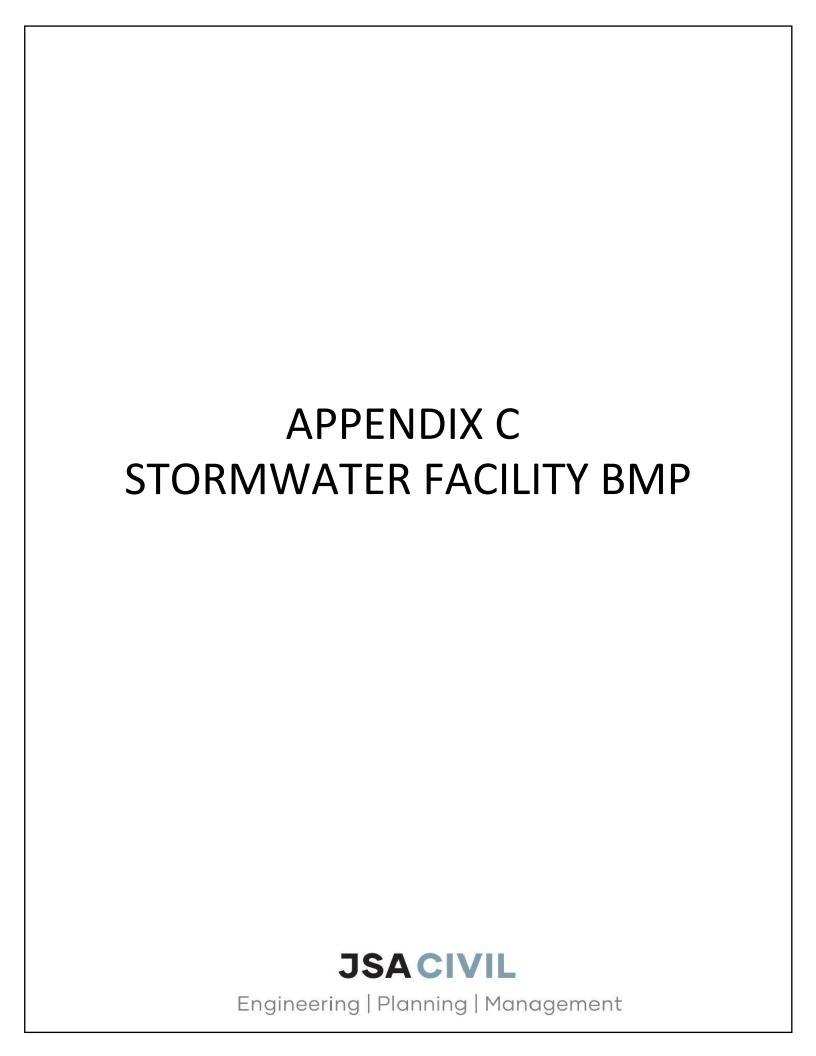
APPENDIX A - VICINITY MAP

APPENDIX B BASIN MAP



BASIN MAP EXHIBIT





APPENDIX D A.3 - STORMWATER MANAGEMENT REPORT CHECKLIST



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APPENDIX A.3 Stormwater Management Report Checklist

The Stormwater Management Report is required for every site improvement where the Presumptive or Performance Approach is used and the minimum submittal requirements are as follows. All reports shall be paginated and securely fastened (including maps and exhibits).

- Engineers scale: Maximum 1"=10' Minimum 1"=50'
- All site topography with existing and proposed contours and spot elevations as necessary
- If there are streets, provide the names
- If curbs and no section view, show the curb height
- Utility structures, underground and overhead lines
- Sidewalks
- Surface materials
- Dimensions

1) Cover Sheet

- Project name and owner
- Site address
- Associated permit numbers
- Submittal date
- Engineer
- Firm
- Address
- Contact information

3) Table of Contents

- 4) Project Overview and Description
 - Size and location of project site (vicinity map)
 - Property zoning
 - Type of development/proposed improvements
 - Watershed description
 - Permits required (local, state, federal)
 - Existing vs. post-construction conditions

5) Methodology

- Drainage at existing site
 - o Potential impacts on the proposed site from existing conditions
 - o Potential impacts from the proposed site on existing drainage
 - o Techniques for mitigating potential conflicts or problems
- Depth to Groundwater Testing results (as applicable)
- Narrative that defines the proposed stormwater management techniques, including discharge point(s) for runoff from private and public impervious areas
- Demonstration of maximized infiltration and vegetative treatment

6) Analysis

- Design Assumptions
 - Design storms used
 - Computation methods
 - Software used
 - Safety factors, curve numbers, and design coefficients
 - Clarify variations from the norm
- Presumptive approach requirements and analysis
- Conveyance requirements and design
- Table of impervious area treated (differentiates public vs. private and roof vs. pavement).
- See example Table 1 below ("Catchment and Facility Table").
- Comparison table of the flow rates for pre and post construction. Table must show that the project meets the flow control requirements set forth in **Section 4.4**. See example Table 2 below ("Pre vs. Post Construction Flow Rates").
- Determination of the escape route or inundation level for the 24-hour 100-year event

Example Table 1

Catchment and Facility Table (shows each catchment on proposed site as well as proposed facility)

Catchment/	Source	Impervious	Ownership	Facility	Facility	Curve
Facility ID	(roof/road/other)	Area	(private/public)	Туре	Size	#
		(sf/ac)			(sf/ac)	
AA						
BB						

Example Table 2

Pre vs. Post Construction Flow Rates

Facility ID		Peak Flow Rate (cfs)							ToC (min)	
	$2 \mathrm{yr}$		$5\mathrm{yr}$ $10\mathrm{yr}$) yr	25 yr				
	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post
Project Site										
AA										
BB										

- 7) Engineering Conclusions
 - Based on compliance with Stormwater Design Manual
 - How water quality, flow control, and discharge requirements are satisfied
 - Post-construction peak flow=pre-development peak flow (2-yr 24-hr events)
- 8) Stormwater Facility Details/Exhibits
 - Contour maps of pre and post development
 - Impervious area identification
 - Watershed delineation
 - Existing and new drainageways
 - Point(s) of discharge
 - Delineation of each catchment (area treated by one facility)
 - Landscape plans (see Appendix D.1)
- 9) Operations and Maintenance Plan and O&M Form (See Appendix A.4)
 - Must include entity responsible for long-term fiscal responsibilities of O&M
- 10) Additional Forms
 - Source Control Special Circumstances Installations (if applicable)
 - Special Circumstances (if applicable)
- 11) Associated Reports Submitted

APPENDIX E A.4 - OPERATIONS AND MAINTENANCE PLAN



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TO BE COMPLETED AT A LATER DATE

Page 1of 3 Form O&M

After Recording Return to: Name: Address:

Place Recording Label Here

APPENDIX A.4 Form O&M: Operations and Maintenance Plan

Permit Application No
Owner Name:
Phone: (area code required)
Mailing Address: (return address for records)
City/State/Zip:
Site Address:
City/State/Zip:
Site Legal Description:
1 Responsible Party for Maintenance (check one) _ Homeowner association _ Property Owner _ Other (describe)
2 Contact Information for Responsible Party(ies) if Other than Owner
Daytime Phone: (area code required)
Instructions

Simplified Sizing Approach: Attach O&M Specifications from the Florence Stormwater Design Manual Appendix H.

Presumptive and Performance Sizing Approach: Attach the site-specific O&M Plan (See Stormwater Design Manual Section 6).

3 Site Plan

Show all facility locations in relation to labeled streets, buildings, or other permanent features on the site. Also show the sources of runoff entering the facility, and the final onsite/offsite discharge point. Please complete the table below

Maintaining the stormwater management facility on this site plan is a required condition of building permit approval for the identified property. The property owner is required to operate and maintain this facility in accordance with the O&M specifications or plan on file with the City of Florence. That requirement is binding on all current and future

Notary Signature:

owners of the property. Failure to comply with the O&M specifications or plan may result in enforcement action, including penalties. The O&M specifications or plan may be modified by written consent of new owners and written approval by re-filing with the Community Development Department.

Complete and record Community Developm Office hours are 8 - 5,	nent Department, 250	Highway 101, Flore			
***************************************	***************************************	***************************************	Required Site Plan (inse	rt here or attach separate she	t)
			☐ I Have A	Attached a Site Plan	
Please complete this table			······		••••
Facility Type	Size (sf)	Drainage is from:	Impervious Area Treated (sf)	Discharge Point	
BY SIGNING BELO executed by filer and reco	f W filer accepts and ag orded with it. To be sign	rrees to the terms and co ned in the presence of a	nditions contained in this O&N notary.	I Form and in any docur	ner
INDIVIDUAL Ackno					
Γhis instrument was a	cknowledged before	me on:			
Ву:					

My Commission Expires: __

CORPORATE Acknowledgement STATE of OREGON county of: This instrument was acknowledged before me on: By: As (title): Of (corporation): Notary Signature:

APPENDIX F SSA REPORT



RAIN GARDEN WITH ADDITIONAL BELOW-GRADE STORAGE

Project Description

File Name

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	Santa Barbara UH
Time of Concentration (TOC) Method	SCS TR-55
Link Routing Method	Hydrodynamic
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	YES

Analysis Options

Start Analysis On	00:00:00	0:00:00
End Analysis On	00:00:00	0:00:00
Start Reporting On	00:00:00	0:00:00
Antecedent Dry Days	0	days
Runoff (Dry Weather) Time Step	0 01:00:00	days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	30	seconds

Number of Elements

	Qty
Rain Gages	1
Subbasins	1
Nodes	2
Junctions	0
Outfalls	1
Flow Diversions	0
Inlets	0
Storage Nodes	1
Links	1
Channels	0
Pipes	1
Pumps	0
Orifices	0
Weirs	0
Outlets	0
Pollutants	0
Land Uses	0

Rainfall Details

SN Rain Gage	Data	Data Source	Rainfall	Rain	State	County	Return	Rainfall	Rainfall
ID	Source	ID	Туре	Units			Period	Depth	Distribution
							(years)	(inches)	
1 RAIN GAGI	Time Series	25-YEAR 24-HOUR EVENT	Cumulative	inches	Oregon	Lane	25.00	5.06	SCS Type IA 24-hr

Subbasin Summary

	SN Subbasin	Area	Impervious	Impervious	Pervious	Total	Total	Total	Peak	Time of
	ID		Area	Area Curve	Area Curve	Rainfall	Runoff	Runoff	Runoff	Concentration
				Number	Number			Volume		
		(ac)	(%)			(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
•	1 DEVELOPED_CONDITIONS	0.97	61.00	98.00	76.00	5.05	3.94	3.82	0.93	0 00:05:00

Node Summary

S	N Element	Element	Invert	Ground/Rim	Initial	Surcharge	Ponded	Peak	Max HGL	Max	Min Time of	Total	Total Time
	ID	Туре	Elevation	(Max)	Water	Elevation	Area	Inflow	Elevation	Surcharge	Freeboard Peak	Flooded	Flooded
				Elevation	Elevation				Attained	Depth	Attained Flooding	Volume	
										Attained	Occurrence		
			(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	(ft) (days hh:mm)	(ac-in)	(min)
	1 Out-01	Outfall	0.00					0.00	0.00				
	2 Stor-01	Storage Node	69.50	72.50	0.00		0.00	0.93	72.00			0.00	0.00

Link Summary

	SN Element	t Element	From	To (Outlet)	Length	Inlet	Outlet	Average	Diameter or	Manning's Pea	C Design Flow	Peak Flow/	Peak Flow	Peak Flow	Peak Flow	Total Time Reported
	ID	Type	(Inlet)	Node		Invert	Invert	Slope	Height	Roughness Flov	Capacity	Design Flow	Velocity	Depth	Depth/	Surcharged Condition
			Node			Elevation	Elevation					Ratio			Total Depth	
															Ratio	
					(ft)	(ft)	(ft)	(%)	(in)	(cfs) (cfs)		(ft/sec)	(ft)		(min)
-	1 Link-01	Pipe	Stor-01	Out-01	10.00	72.25	69.50	27.5000	18.000	0.0150 0.0	47.74	0.00	0.00	0.00	0.00	0.00 Calculated

Subbasin Hydrology

Subbasin: DEVELOPED_CONDITIONS

Input Data

Area (ac)	0.97	
Impervious Area (%)	61	
Impervious Area Curve Number	98	
Pervious Area Curve Number	76	
Rain Gage ID	RAIN	GAGE

Composite Curve Number

32	Area	Soil	Curve
Soil/Surface Description	(acres)	Group	Number
Composite Area & Weighted CN	0.97		89.42

Time of Concentration

TOC Method : SCS TR-55

Sheet Flow Equation :

 $Tc = (0.007 * ((n * Lf)^0.8)) / ((P^0.5) * (Sf^0.4))$

Where:

Tc = Time of Concentration (hr)

n = Manning's roughness

Lf = Flow Length (ft)

P = 2 yr, 24 hr Rainfall (inches)

Sf = Slope (ft/ft)

Shallow Concentrated Flow Equation:

V = 16.1345 * (Sf^0.5) (unpaved surface)

V = 20.3282 * (Sf^0.5) (paved surface)

V = 15.0 * (Sf^0.5) (grassed waterway surface)

V = 10.0 * (Sf^0.5) (nearly bare & untilled surface)

 $V = 9.0 * (Sf^0.5)$ (cultivated straight rows surface)

V = 7.0 * (Sf^0.5) (short grass pasture surface)

 $V = 5.0 * (Sf^0.5)$ (woodland surface)

V = 2.5 * (Sf^0.5) (forest w/heavy litter surface)

Tc = (Lf / V) / (3600 sec/hr)

Where:

Tc = Time of Concentration (hr)

Lf = Flow Length (ft)

V = Velocity (ft/sec)

Sf = Slope (ft/ft)

Channel Flow Equation :

 $V = (1.49 * (R^{(2/3)}) * (Sf^{0.5})) / n$

R = Aq / Wp

Tc = (Lf / V) / (3600 sec/hr)

Where:

Tc = Time of Concentration (hr)

Lf = Flow Length (ft)

R = Hydraulic Radius (ft)

Aq = Flow Area (ft²)

Wp = Wetted Perimeter (ft)

V = Velocity (ft/sec)

Sf = Slope (ft/ft)

n = Manning's roughness

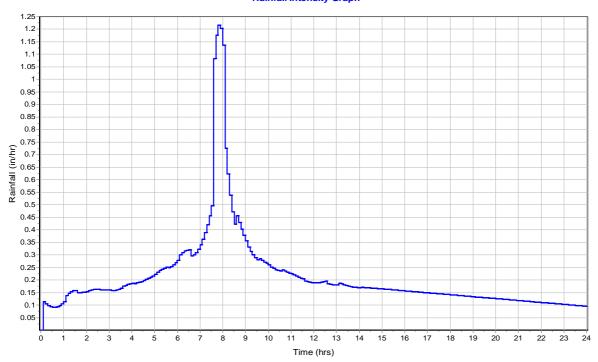
User-Defined TOC override (minutes): 5.00

Subbasin Runoff Results

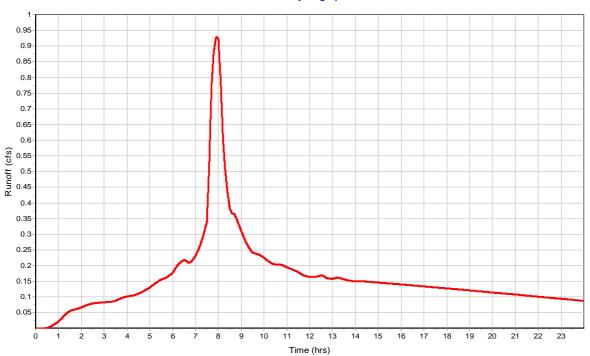
Total Rainfall (in)	5.05
Total Runoff (in)	3.94
Peak Runoff (cfs)	0.93
Weighted Curve Number	89.42
Time of Concentration (days hh:mm:ss)	0 00:05:00

Subbasin: DEVELOPED_CONDITIONS

Rainfall Intensity Graph



Runoff Hydrograph



Pipe Input

SN Element	Length	Inlet	Inlet	Outlet	Outlet	Total	Average Pipe	Pipe	Pipe	Manning's	Entrance	Exit/Bend	Additional	Initial Flap	No. of
ID		Invert	Invert	Invert	Invert	Drop	Slope Shape	Diameter or	Width	Roughness	Losses	Losses	Losses	Flow Gate	Barrels
		Elevation	Offset	Elevation	Offset			Height							
	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(%)	(in)	(in)					(cfs)	
1 Link-01	10.00	72.25	2.75	69.50	69.50	2.75	27.5000 CIRCULAR	18.000	18.000	0.0150	0.5000	0.5000	0.0000	0.00 No	1

Pipe Results

S	N Element	Peak	Time of	Design Flow	Peak Flow/	Peak Flow	Travel	Peak Flow	Peak Flow	Total Time	Froude Reported
	ID	Flow	Peak Flow	Capacity	Design Flow	Velocity	Time	Depth	Depth/	Surcharged	Number Condition
			Occurrence		Ratio				Total Depth		
									Ratio		
		(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		(min)	
	1 Link-01	0.00	0 00:00	47.74	0.00	0.00		0.00	0.00	0.00	Calculated

Storage Nodes

Storage Node: Stor-01

Input Data

Invert Elevation (ft)	69.50
Max (Rim) Elevation (ft)	72.50
Max (Rim) Offset (ft)	3.00
Initial Water Elevation (ft)	0.00
Initial Water Depth (ft)	-69.50
Ponded Area (ft²)	0.00
Evaporation Loss	0.00

Infiltration/Exfiltration

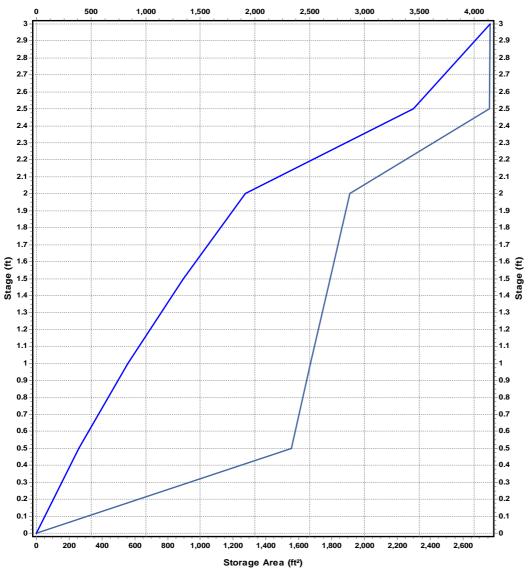
Constant Flow Rate (cfs) 0.15

Storage Area Volume Curves
Storage Curve: RAIN GARDE W/ BELOW-GRADE STORAGE

Stage	Storage	Storage
	Area	Volume
(ft)	(ft²)	(ft³)
0	0	0
0.5	1556	389
1	1670	835
1.5	1789.33	1342
2	1911	1911
2.5	2760	3450
3	2766.67	4150

Storage Area Volume Curves





Storage Node : Stor-01 (continued)

Output Summary Results

Peak Inflow (cfs)	0.93
Peak Lateral Inflow (cfs)	0.93
Peak Outflow (cfs)	0
Peak Exfiltration Flow Rate (cfm)	9
Max HGL Elevation Attained (ft)	72
Max HGL Depth Attained (ft)	2.5
Average HGL Elevation Attained (ft)	71.16
Average HGL Depth Attained (ft)	1.66
Time of Max HGL Occurrence (days hh:mm)	0 14:20
Total Exfiltration Volume (1000-ft³)	11.456
Total Flooded Volume (ac-in)	0
Total Time Flooded (min)	0
Total Retention Time (sec)	0